

C.C. Tatham & Associates Ltd. Consulting Engineers

WASAGA SHORES SUBDIVISION

Town of Wasaga Beach

Shoreline Hazard Study

prepared by:

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TABLE OF CONTENTS

1	Introduction	1
1.1	Objectives	1
1.2	Guidelines & Background Information	1
2	Existing Shoreline Conditions	2
2.1	Existing Conditions	2
2.2	Flood Hazard	2
2.3	Wave Uprush Calculations	2
2.4	Erosion	3
2.5	Ice Piling	3
2	Conclusions & Recommendations	Д
J		-1

APPENDICES

Appendix A: Site Photographs Appendix B: Wave Uprush Calculations Appendix C: Erosion

1 Introduction

C.C. Tatham & Associates Ltd. (CCTA) has prepared a Shoreline Hazard Study in support of a plan of subdivision application at 2320 Shore Lane in the Town of Wasaga Beach (Town). The subject property is regulated by the Nottawasaga Valley Conservations Authority (NVCA) as it is located along the shoreline of Georgian Bay, and is therefore subject to the associated flood and erosion hazards. Approval must be obtained from NVCA to allow for approval of the proposed draft plan of subdivision.

The proposed development will consist of a road connecting Betty Boulevard and Constance Boulevard, and 22 lots with two of the lots fronting Georgian Bay. A key plan is provided as **Figure 1**.

1.1 **Objectives**

The primary objective of this report is to demonstrate that the siting of the proposed development has made appropriate consideration for shoreline hazards. This will be accomplished by determining the extent of shoreline hazards acting on the subject shoreline and establishing development setbacks and minimum building elevations as required.

1.2 Guidelines & Background Information

This report was prepared recognizing provincial guidelines on natural hazards, including the following publications:

- The Ministry of the Natural Resources (now MNRF) Technical Guide for Great Lakes St. Lawrence River Shorelines (2001);
- NVCA Natural Hazards Technical Guide (2013); and
- Environment Canada and MNR Coastal Zone Atlas Great Lakes Shore Damage Survey (1973).

Information relating to existing topography, ground cover and shoreline conditions was obtained through a review of relevant background studies, available plans and base mapping.

2 Existing Shoreline Conditions

2.1 Existing Conditions

The property is located on Georgian Bay. The site is exposed to waves generated from the northwestnorth-northeast directions. The subject shoreline is a sand beach that transitions to mature vegetation further from shore, at a slope of approximately 2.0%. A series of cobble/boulder shoreline revetments exist on neighbouring properties.

The subject shoreline is located in a transitional area between sections of depositional sand beach and riprap beach per Environment Canada's *Environmental Sensitivity Atlas for Lake Huron's Canadian Shoreline* (1994). Shore-zone sediment transport occurs in a south-east direction.

The existing site is undeveloped and consists of forested land cover. Site photographs of the existing site conditions are seen in **Appendix A**.

2.2 Flood Hazard

The flood hazard limit is defined in the Provincial Policy Statement as the 100-year flood level plus a wave uprush allowance. The 100-year flood elevation applicable to this portion of the Georgian Bay shoreline is 178.0 m and includes the static lake level and wind setup, per the MNRF Technical Guide for Great Lakes – St. Lawrence River Shorelines.

We note that there is a small depression at the rear of the two waterfront lots that is just below 178.0 m, however, it is separated from the lake by a continuous ridge of higher ground. Thus, the small depression is isolated and not part of the flood hazard.

2.3 Wave Uprush Calculations

To determine the design wave uprush elevation, CCTA carried out an extreme value analysis of offshore wave conditions. Using historical wave data from buoy C45143 located in Georgian Bay, it was determined that the significant wave height with a 1:100-year return period is 6.6 m. This wave height was transformed to the nearshore using the SWAN spectral wave model with 300 m resolution that CCTA has developed to simulate wave conditions across Southern Georgian Bay. For a location offshore of the subject site, the SWAN model returned a wave height of 1.57 m with a corresponding wave period of 4.1 seconds.

The calculated wave height was then transformed to the shoreline to produce a wave run-up value using design charts from the United States Army Corps of Engineering Research Centre. The wave run-up was calculated to be 0.20 m, with the waves breaking on the beach. The resulting flood hazard elevation is 178.20 m. Supporting calculations are found in **Appendix B**.

2.4 Erosion

The subject sand beach is not belived to be a dynamic beach. Historic photos from 1978, 1989, 1997, 2008 and 2016 show an unchanging established treeline at the site location, therefore there is no dynamic beach and no dynamic beach hazard. Historic photos are attached in **Appendix C**.

Additionally, the Coastal Zone Atlas was used to confirm the likely extent of erosion at the site location. The site is located between erosion monitoring stations H-32 and H-33. The shoreline is described as a beach or dune complex, with permanent residential land use. No shoreline damage due to erosion is described at the site location.

The nearest erosion station to the subject site is H-32, which shows a recession rate of 0.0 m/year from 1954 to 1973. The station is located approximately 1.0 km east of the subject site. The closest station to the west is H-33, located approximately 2.0 km from the site. The station shows an accession rate of 0.0 m/year from 1954 to 1972.

It is our opinion, based on review of historic aerial photos and available erosion station records, that the average annual recession rate is negligible and there is no erosion hazard at the subject site. The relevant pages from the Costal Zone Atlas are found attached in **Appendix C**.

2.5 Ice Piling

Wind can cause melting ice to pile along shorelines and be pushed landward, causing erosion and damage to structures that are encountered.

A 15 m setback, in which the existing shoreline vegetation is preserved, is recommended to provide appropriate protection from hazards associated with ice piling. In addition, a minimum opening elevation of 178.6 m should be applied to all structures.

The hazard limits and proposed setbacks are shown on Drawing HAZ-1.

3 Conclusions & Recommendations

The proposed 22 lot subdivision can be protected from shoreline hazards through the application of a 15 m setback from the 100-year flood elevation of 178.0 m. This 15 m setback extends further landward than the calculated wave uprush elevation of 178.20 m and allows for potential erosion and ice piling effects. In addition, all proposed habitable structures should be constructed with a minimum opening elevation of 178.6 m.

All proposed building envelopes are to be located outside the shoreline hazard limit, as shown in **Drawing HAZ-1**. The shoreline hazard setback is to be enforceable as a provision of the by-law.

We trust the above presentation of a Shoreline Hazard Study is sufficient to satisfy the Draft Plan of Subdivision requirements.

All of which is respectfully submitted, C.C. Tatham & Associates Ltd.

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1.	ISSUED FOR NVCA APPROVAL	NOV 15/18	ALK	APPROVED	WASAGA SHORES PART OF LOTS 34 AND 33 PART 11, 12 & 13, PL TOWN OF WASAG COUNTY OF S SHORELINE HAZ
NO.	REVISIONS	DATE	INITIAL		



<u>LEGEND</u>

PROPERTY/LOT LINE EXISTING CENTERLINE EXISTING EDGE OF ASPHALT EXISTING STORM SEWER EXISTING STORM MAINTENANCE HOLE PROPOSED REAR-YARD BIOSWALE PROPOSED SWALE PROPOSED GRADE & DIRECTION PROPOSED/EXISTING GRADES EXISTING FLOW DIRECTION

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2320 Shore Lane, Town of Wasaga Beach Key Plan Figure

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APPENDIX A: SITE PHOTOGRAPHS



Photograph 1: Existing Shoreline Facing North West



Photograph 2: Existing Shoreline and Vegetation Limit Facing East



Photograph 3: Neighbouring Shoreline Protection (Facing North West)



Photograph 4: Neighbouring Shoreline Protection (Facing East)

APPENDIX B: WAVE UPRUSH CALCULATIONS



APPENDIX C: EROSION







Site Photo - 1997



Site Photo - 2008









